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Extended mild-slope equation for random waves

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Abstract

A time-dependent extended mild-slope equation is derived from the elliptic equation of Chamberlain and Porter [J. Fluid Mech. 291 (1995) 393] using the Taylor series technique. Numerical tests are made on a horizontally one-dimensional case for regular waves over sloping beds and for both regular and irregular waves over a ripple patch. Numerical results prove that the proposed model gives accurate results for both regular and irregular waves over rapidly varying topography.

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1. Introduction

Waves generated in deep water propagate to shallow water undergoing various forms of transformations, like shoaling, refraction, diffraction, and reflection, etc. Since the development of a mild-slope equation by Berkhoff (1972), many mathematical models have been developed to analyze the combined refraction and diffraction of waves. In order to model the propagation of random waves, time-dependent mild-slope equations have also been developed. Smith and Sprinks (1975) developed a hyperbolic mild-slope

equation using Green's formula. Radder and Dingemans (1985) proposed a canonical form of the time-dependent mild-slope equations based on the Hamiltonian theory of surface waves. Kubo et al. (1992) also developed another type of time-dependent mild-slope equation using the Taylor series expansion technique for waves with local frequencies different from the carrier frequency. Lee and Kirby (1994) analyzed the mild-slope equations developed by Smith and Sprinks, and Kubo et al. in terms of the dispersion relation and energy transport. Considering the dispersion relation, Smith and Sprinks' model is more accurate in shallower water ($kh \leq 0.2\pi$), whereas Kubo et al.'s model is more accurate in deeper water ($kh > 0.2\pi$). Kubo et al.'s model is more accurate for energy transport but has a singularity problem in the higher frequency range.

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Nishimura et al. (1983) developed hyperbolic mild-slope equations by vertically integrating the continuity and momentum equations for linear waves. Starting from the time-dependent mild-slope equation of Smith and Sprinks, Copeland (1985) developed similar hyperbolic equations using the characteristics of linear waves and the defined volume flux. The two models of Nishimura et al. and Copeland predict the transformation of regular waves.

In the derivation of the mild-slope equations, an assumption of $|\nabla h|/kh \ll 1$ (where ∇ = horizontal gradient operator, k = wave number, and h = water depth) was made so that the terms of the second-order bottom effect related to the bottom curvature and the square of the slope were neglected. To account for the effect of a steeply sloping and rapidly varying bottom topography, Massel (1993) and Chamberlain and Porter (1995) derived the extended mild-slope equation using the Galerkin-eigenfunction method. Chandrasekera and Cheung (1997) provided an alternative derivation of the extended equation based on the approach of Berkhoff. Suh et al. (1997) used the Green's second identity and Lagrangian formula to develop two ultimately equivalent hyperbolic equations for random waves. Lee et al. (1998) recast the elliptic formulation of Massel into a hyperbolic formulation, following the technique of Copeland.

In this study, by applying Kubo et al.'s method to the equation of Chamberlain and Porter, we developed a time-dependent version of the extended mild-slope equation, which may be able to predict the transformation of random waves on rapidly varying topography. In order to verify the improved accuracy of our equation compared to the equation of Kubo et al. in the case of a rapidly varying topography, the two models were initially applied to the problem of wave reflection from a planar slope with different inclinations (Booij, 1983). Subsequently, the equation was applied to the problem of the resonant Bragg reflection of monochromatic waves from a ripple patch (Davies and Heathershaw, 1984). Finally, to investigate the applicability of the proposed equation to random waves, numerical tests were performed on the transmission of unidirectional random waves normally incident to a finite ripple patch.

2. Development of governing equation

The extended mild-slope equation of Chamberlain and Porter (1995) is given by

$$\nabla \cdot (CC_g \nabla \eta) + \{k^2 CC_g + gu_1 \nabla^2 h + gu_2 (\nabla h)^2\} \eta = 0 \quad (1)$$

where the coefficients u_1 and u_2 determining the second-order bottom effects are given by

$$u_1 = \frac{\text{sech}^2 kh}{4(2kh + \sinh 2kh)} (\sinh 2kh - 2kh \cosh 2kh) \quad (2)$$

$$u_2 = \frac{k \text{sech}^2 kh}{12(2kh + \sinh 2kh)^3} \{ (2kh)^4 + 4(2kh)^3 \sinh 2kh - 9 \sinh 2kh \sinh 4kh + 6kh(2kh + 2 \sinh 2kh) \times (\cosh^2 2kh - 2 \cosh 2kh + 3) \} \quad (3)$$

and η is the water surface elevation, C and C_g are the phase speed and group velocity, respectively, of a wave with an angular frequency ω , and g is the gravitational acceleration. The wave number k is determined from the linear dispersion relation for Stokes waves.

For random waves, the surface elevation is defined as the superposition of component waves

$$\eta = \text{Re} \left[\sum_{m=1}^{\infty} \eta_m e^{-i\omega_m t} \right] \quad (4)$$

where ω_m and η_m are the angular frequency and complex amplitude, respectively, of the m th component wave. In order to apply the extended mild-slope equation to random waves, we define a variable $\hat{\eta}$ as

$$\eta = \text{Re}[\hat{\eta} e^{-i\hat{\omega} t}] \quad (5)$$

where $\bar{\omega}$ is a carrier angular frequency of the random waves. From Eqs. (4) and (5), we have

$$\frac{\partial \hat{\eta}_m}{\partial t} = -i\Delta\omega_m \hat{\eta}_m \quad (6)$$

where $\Delta\omega_m = \omega_m - \bar{\omega}$. The variable of component wave, $\hat{\eta}_m$, is a solution of Eq. (1), as well as η_m . Therefore, Eq. (1) can be rewritten in terms of $\hat{\eta}_m$. If we expand the quantities $(CC_g)_m$, $(k^2CC_g)_m$, $(u_1)_m$, and $(u_2)_m$ into the Taylor series of $\Delta\omega_m$ and use Eq. (6), we can obtain an equation in which all the variables can be determined by the coefficients of the carrier frequency only. Superposition of the resulting equations for all the component waves yields the following equation

$$\begin{aligned} \nabla \cdot (\bar{C}\bar{C}_g \nabla \hat{\eta}) + \{\bar{k}^2 \bar{C}\bar{C}_g + g\bar{u}_1 \nabla^2 h + g\bar{u}_2 (\nabla h)^2\} \hat{\eta} \\ + i \nabla \cdot \left\{ \frac{\partial (CC_g)}{\partial \omega} \nabla \frac{\partial \hat{\eta}}{\partial t} \right\} + i \left\{ \frac{\partial (k^2 CC_g)}{\partial \omega} \right. \\ \left. + g \frac{\partial \bar{u}_1}{\partial \omega} \nabla^2 h + g \frac{\partial \bar{u}_2}{\partial \omega} (\nabla h)^2 \right\} \frac{\partial \hat{\eta}}{\partial t} = 0 \end{aligned} \quad (7)$$

where the over bar indicates the variables associated with the carrier angular frequency $\bar{\omega}$ and the coefficients are given by

$$\begin{aligned} \frac{\partial (CC_g)}{\partial \omega} = \frac{\bar{\omega}}{\bar{k}^2} \left[2(\bar{n} - 1) + \frac{2\bar{n} - 1}{2\bar{n}} \right. \\ \left. \times \{1 - (2\bar{n} - 1)\cosh 2\bar{k}h\} \right] \end{aligned} \quad (8)$$

$$\begin{aligned} \frac{\partial (k^2 CC_g)}{\partial \omega} \\ = \bar{\omega} \left[2\bar{n} + \frac{2\bar{n} - 1}{2\bar{n}} \{1 - (2\bar{n} - 1)\cosh 2\bar{k}h\} \right] \end{aligned} \quad (9)$$

$$\begin{aligned} \frac{\partial \bar{u}_1}{\partial \omega} = - \frac{\bar{k}h \operatorname{sech}^2 \bar{k}h}{2\bar{\omega}\bar{n}(2\bar{k}h + \sinh 2\bar{k}h)} \left\{ 2\bar{k}h \sinh 2\bar{k}h \right. \\ \left. + \left(\tanh \bar{k}h + \frac{1 + \cosh 2\bar{k}h}{2\bar{k}h + \sinh 2\bar{k}h} \right) \right. \\ \left. \times (\sinh 2\bar{k}h - 2\bar{k}h \cosh 2\bar{k}h) \right\} \end{aligned} \quad (10)$$

$$\begin{aligned} \frac{\partial \bar{u}_2}{\partial \omega} = \frac{\bar{k} \operatorname{sech}^2 \bar{k}h}{12\bar{\omega}\bar{n}(2\bar{k}h + \sinh 2\bar{k}h)^3} \\ \times \left[\left\{ 1 - 2\bar{k}h \tanh \bar{k}h - \frac{6\bar{k}h(1 + \cosh 2\bar{k}h)}{(2\bar{k}h + \sinh 2\bar{k}h)} \right\} \right. \\ \times \left\{ (2\bar{k}h)^4 + 4(2\bar{k}h)^3 \sinh 2\bar{k}h \right. \\ - 9\sinh 2\bar{k}h \sinh 4\bar{k}h + 12\bar{k}h(\bar{k}h + \sinh 2\bar{k}h) \\ \left. \times (\cosh^2 2\bar{k}h - 2\cosh 2\bar{k}h + 3) \right\} + \bar{k}h \left\{ 8(2\bar{k}h)^3 \right. \\ + 24(2\bar{k}h)^2 \sinh 2\bar{k}h + 8(2\bar{k}h)^3 \cosh 2\bar{k}h \\ - 18\cosh 2\bar{k}h \sinh 4\bar{k}h - 36\sinh 2\bar{k}h \cosh 4\bar{k}h \\ + 12(2\bar{k}h + \sinh 2\bar{k}h + 2\bar{k}h \cosh 2\bar{k}h)(\cosh^2 2\bar{k}h \\ - \cosh 2\bar{k}h + 3) + 48\bar{k}h \sinh 2\bar{k}h(\bar{k}h + \sinh 2\bar{k}h) \\ \left. \left. \times (\cosh 2\bar{k}h - 1) \right\} \right] \end{aligned} \quad (11)$$

$$\bar{n} = \frac{\bar{C}_g}{\bar{C}} = \frac{1}{2} \left(1 + \frac{2\bar{k}h}{\sinh 2\bar{k}h} \right) \quad (12)$$

Eq. (7) is a time-dependent extended mild-slope equation for random waves. If $\bar{u}_1 = \bar{u}_2 = 0$, Eq. (7) reduces to the equation of Kubo et al. (1992).

3. Comparison of the proposed equation with Suh et al.'s equation

The proposed equation and Suh et al.'s (1997) equation are known to accurately predict waves on rapidly varying topography. The accuracy of the equations is due to the inclusion of the second-order bottom effect terms. In this section, the terms of higher-order bottom effect are compared between the two equations. The two equations are also known to be capable of predicting transformation of random waves with a narrow frequency band. So, the applicability of the two models to random waves is investigated in terms of the dispersion relation following the geometric optics approach of Lee and Kirby (1994).

Using the relation of $\partial \tilde{\phi} / \partial t = -g\eta$, where $\tilde{\phi}$ is the velocity potential at the mean water level, the equation of Suh et al. can be written in terms of η as:

$$\begin{aligned} \frac{\partial^2 \eta}{\partial t^2} - \nabla \cdot (\bar{C}\bar{C}_g \nabla \eta) + (\bar{\omega}^2 - \bar{k}^2 \bar{C}\bar{C}_g) \eta \\ + \bar{\omega}^2 (\bar{R}_1 (\nabla h)^2 + \bar{R}_2 \nabla^2 h) \eta = 0 \end{aligned} \quad (13)$$

where the parameters $\bar{\omega}^2 \bar{R}_1$ and $\bar{\omega}^2 \bar{R}_2$ determining the second-order bottom effects are mathematically equivalent to $-g\bar{u}_2$ and $-g\bar{u}_1$, respectively, of the proposed Eq. (7). These higher-order bottom effects were studied in detail by Lee et al. (1998).

The geometric optics approach is used to get the dispersion relations of the proposed equation and Suh et al.'s equation. Let the surface elevation η be

$$\eta = a(x,y)\exp(i\psi) \tag{14}$$

where $a(x,y)$ is a complex amplitude which modulates in space, and the phase function ψ has the following relation with the local wave number k and angular frequency ω as:

$$k = \nabla\psi, \quad \omega = -\frac{\partial\psi}{\partial t} \tag{15}$$

Then the variable $\hat{\eta}$ in Eq. (7) may be written as:

$$\hat{\eta} = a(x,y)\exp[i(\psi + \bar{\omega}t)] \tag{16}$$

Substituting Eq. (16) into Eq. (7) and neglecting both the higher-order bottom effects and diffraction effects, the real part gives the dispersion relation:

$$\frac{k}{\bar{k}} = \sqrt{1 + \frac{2\left(\frac{\omega}{\bar{\omega}} - 1\right)}{\frac{\bar{C}_g}{\bar{C}} + \left(\frac{\omega}{\bar{\omega}} - 1\right) \frac{\bar{\omega}}{\bar{C}^2} \frac{\partial}{\partial \omega} (CC_g)}} \tag{17}$$

On the other hand, substituting Eq. (14) into the Suh et al.'s Eq. (13) and again neglecting those effects, the real part gives the dispersion relation:

$$\frac{k}{\bar{k}} = \sqrt{1 + \frac{\bar{C}}{\bar{C}_g} \left[\left(\frac{\omega}{\bar{\omega}}\right)^2 - 1 \right]} \tag{18}$$

These dispersion relations can be compared with the exact one for a linear wave:

$$\frac{k}{\bar{k}} = \left(\frac{\omega}{\bar{\omega}}\right)^2 \frac{\tanh \bar{k}h}{\tanh kh} \tag{19}$$

These dispersion relations are dependent on the relative wave depth ($\bar{k}h$) corresponding to the carrier frequency. Fig. 1 shows the dispersion relations in a shallow water ($\bar{k}h=0.05\pi$) and in a deep water ($\bar{k}h=2\pi$). In the case of shallow water, the dispersion

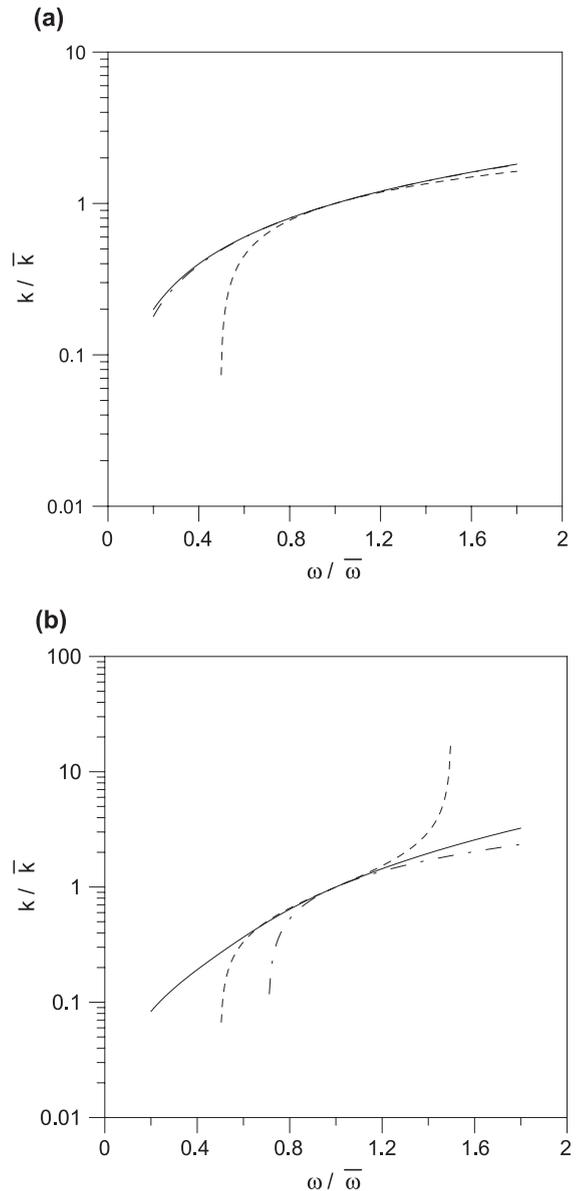


Fig. 1. Dispersion relation for (a) $\bar{k}h=0.05\pi$ and (b) $\bar{k}h=2.0\pi$; solid line=exact, dashed line=proposed model, dash-dotted line=Suh et al.'s model.

relation of the proposed model is close to the exact one at higher frequencies but diverges at lower frequencies. However, the agreement between the dispersion relation of Suh et al.'s model and the exact one is excellent. For deep water, the dispersion relation of the proposed model is closer to the exact

one than Suh et al.'s model at lower frequencies. However, the dispersion relation of Suh et al.'s model is closer to the exact one at higher frequencies.

The relative water depth is obtained such that there is a minimal squared relative error in the dispersion relation of the proposed model compared to Suh et al.'s model. The squared relative error is defined as:

$$E(\bar{k}h) = \int_{\frac{\omega}{\bar{\omega}}=0.2}^{1.8} \left(\frac{k_s - k_p}{k_s} \right)^2 d\left(\frac{\omega}{\bar{\omega}}\right) \quad (20)$$

where k_s is the wave number obtained by Eq. (18) and k_p is the wave number obtained by Eq. (17). The squared relative errors are found in the depth range of $\bar{k}h = 0.05\pi - 1.00\pi$. At the relative water depth of $\bar{k}h = 0.247\pi$, the squared relative error is minimized, i.e. the dispersion relations of the two models are almost equal to each other. Fig. 2 shows the dispersion relations when the relative water depth is 0.247π . The two dispersion relations are almost equal to each other except in the higher frequency range.

For a linear dispersion, Suh et al.'s model is more accurate than the proposed model when the relative water depth is shallower than 0.247π . When the relative water depth is deeper than 0.247π , the proposed model shows better accuracy at lower frequen-

cies, and Suh et al.'s model is more accurate at higher frequencies.

In shallow water, the nonlinearity of waves becomes prominent and thus both the proposed model and Suh et al.'s model, which are linear, cannot yield accurate solutions for the dispersion relation. In reality, the incident wave spectrum has little energy at higher frequencies, and most of the wave energy is in the lower frequency range. Therefore, the accuracy of the dispersion relation at lower frequencies determines the accuracy of the major change of a wave energy spectrum. This tendency gives an advantage to the proposed model compared to Suh et al.'s model.

4. Numerical experiments

To examine the accuracy of the proposed model, we conducted numerical experiments with both the proposed model and Kubo et al.'s (1992) model for monochromatic waves propagating over a planar slope using different inclinations of Booij (1983). The reflection coefficients calculated by the models were compared against those calculated by the finite element method (Suh et al., 1997). The two models were also applied to the case of a wave propagating over the ripple patch of Davies and Heathershaw (1984). The calculated reflection coefficients were compared against the experimental data. Finally, the two models and the model of Suh et al. (1997) were tested for random waves propagating over the ripple patch. The solutions were compared against the finite element model solution, which can be regarded as an exact solution for linear random waves.

4.1. Finite difference method

Internal generation of waves has been used widely in generating waves in time-dependent wave equations. Using the viewpoint of mass transport, Larsen and Dancy (1983) employed a line source to generate waves in the staggered grid solution of the Boussinesq equations. Lee and Suh (1998), and Lee et al. (2001) found that the internal generation of waves would be more accurate using the viewpoint of energy transport. However, the line source method may cause a problem for unstaggered grid systems. For this reason, Wei et al. (1999) generated waves using the source func-

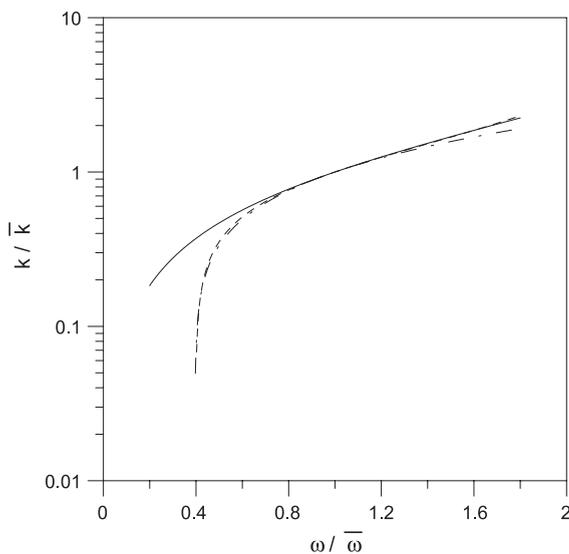


Fig. 2. Dispersion relation for $\bar{k}h = 0.247\pi$; solid line = exact, dashed line = proposed model, dash-dotted line = Suh et al.'s model.

tion method in the unstaggered grid solution of the Boussinesq equations. It is not possible to get a staggered grid solution for the proposed model. Therefore, we adopted a source function method to generate the waves.

Sponge layers were placed at the outside boundaries to remove wave reflection from the boundaries by dissipating wave energy inside the sponge layers. The proposed model Eq. (7) is therefore modified as:

$$\begin{aligned} & \nabla \cdot (\bar{C}\bar{C}_g \nabla \hat{\eta}) + \left\{ \bar{k}^2 \bar{C}\bar{C}_g + g\bar{u}_1 \nabla^2 h + g\bar{u}_2 (\nabla h)^2 \right\} \\ & \times \left(1 + i \frac{D_s}{\bar{n}} \right) \hat{\eta} + i \nabla \cdot \left\{ \frac{\partial (\bar{C}\bar{C}_g)}{\partial \omega} \nabla \frac{\partial \hat{\eta}}{\partial t} \right\} \\ & + i \left\{ \frac{\partial (\bar{k}^2 \bar{C}\bar{C}_g)}{\partial \omega} + g \frac{\partial \bar{u}_1}{\partial \omega} \nabla^2 h + g \frac{\partial \bar{u}_2}{\partial \omega} (\nabla h)^2 \right\} \\ & \times \left(1 + i \frac{D_s}{\bar{n}} \right) \frac{\partial \hat{\eta}}{\partial t} = f(x, y, t) \end{aligned} \quad (21)$$

where $f(x,y,t)$ is a source function and the damping coefficient D_s is given by

$$D_s = \begin{cases} 0, & \text{outside sponge layer} \\ \frac{\exp(d/S) - 1}{\exp(1) - 1}, & \text{inside sponge layer} \end{cases} \quad (22)$$

where d is the distance from the starting point of the sponge layer and S is the thickness of the sponge layer. In a one-dimensional domain, the modified model is discretized by the Crank–Nicolson method

in time and space. Initially, surface elevations are set to zero. In order to generate waves gradually, the source function is multiplied by $\tanh(t/p\bar{T})$ where \bar{T} is the carrier wave period and p is a parameter corresponding to the rate of slow wave generation. Since the sponge layer reduces the incoming wave energy effectively, the reflective condition at the boundaries can be used.

4.2. Wave reflection of monochromatic waves from a planar slope

We conducted numerical experiments with both the proposed model and Kubo et al.’s model for monochromatic waves propagating over a planar slope, each end of which was connected to a constant-depth region, as tested by Booij (1983), who investigated the accuracy of the mild-slope equation with respect to bottom slope. Since Booij’s tests, many researchers have adopted this numerical experiment to demonstrate accuracy of their model. The computational domain for the numerical test is shown in Fig. 3. The water depths on the up-wave and down-wave sides of the slope are $h_1=0.6$ m and $h_2=0.2$ m, respectively, and the width of the slope, b , is varied so that the inclination of the slope varies. The wave period is 2 s. The grid size used is $\Delta x=0.05$ m, and the time step is $\Delta t=0.05$ s.

Fig. 4 shows the reflection coefficients calculated by the proposed equation, the Kubo et al.’s equation, and the finite element model of Suh et al. The finite element model solution may be regarded as an exact solution. It is shown that the proposed model gives very close answers to those of the finite element

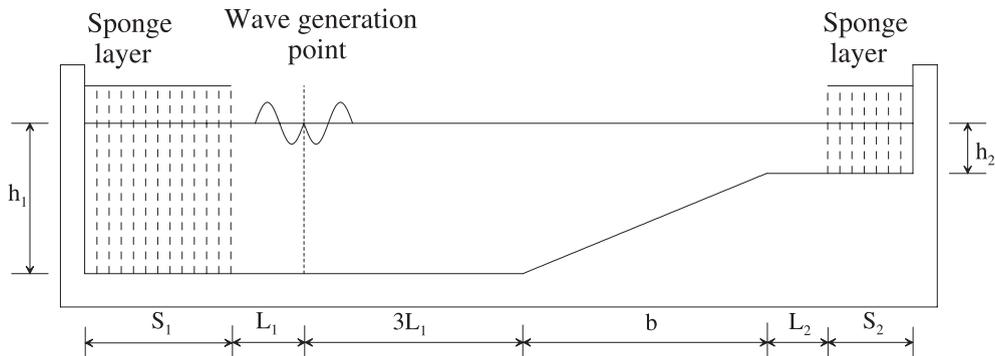


Fig. 3. Computational domain for numerical test of waves propagating over a planar slope.

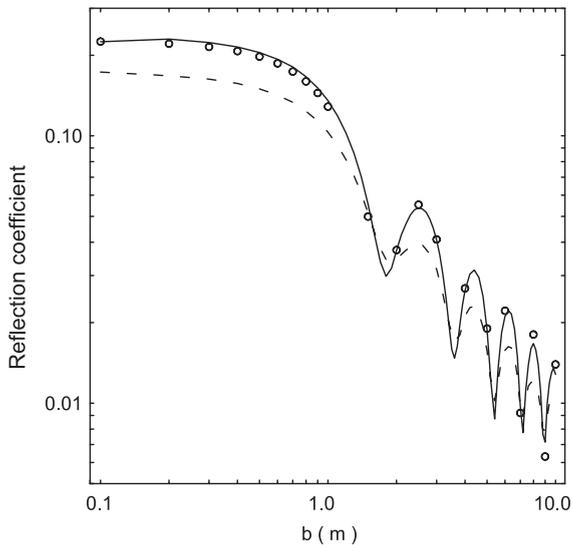


Fig. 4. Reflection coefficient vs. width of a plane slope; dashed line=Kubo et al.'s model, solid line=proposed model, circle=finite element model.

model and the reflection coefficient becomes stable even for very steep slopes. Conversely, Kubo et al.'s model under-predicts the reflection coefficient for steeper slopes. Even for very mild slopes, the proposed model, and Kubo et al.'s model show some differences and the results of the finite element model coincide with those of the proposed model rather than the Kubo et al.'s model.

Recently, Lee et al. (1998) indicated that bed topography is not sufficient to determine the accuracy of a wave equation with respect to the bottom slope, because the water depth tested is only in water of immediate depth and the two slope discontinuities affect the solution. The slope discontinuities are taken into account in the extended mild-slope equation through the bottom curvature terms.

4.3. Bragg reflection of monochromatic waves from a ripple patch

When surface waves are normally incident over a ripple patch, a significant portion of the wave energy is reflected from the ripple patch if the wavelength of the surface wave is twice that of the ripple. This wave reflection is called the resonant Bragg reflection.

Davies and Heathershaw reported a series of experimental data for different numbers of ripples and water depths. In their experiment, the ripple wavelength and amplitude were 1 m and 5 cm, respectively, and the numbers of ripples were 2, 4, and 10. The water depth in the constant-depth region was 15.6 cm for the case of 2 and 4 ripples and 31.3 cm for the case of 10 ripples. These experimental data have been used for comparison with various numerical models by a number of researchers. All of them showed that the mild-slope equation gives a good agreement with the experimental data for the cases of 2 and 4 ripples, but for the case of 10 ripples, it fails to predict the magnitude of the Bragg reflection. Therefore, in this study, a numerical test was made only for the case of 10 ripples. The water depth in the computational domain is given by

$$h(x) = \begin{cases} h_c, & x \leq x_r \\ h_c - A \sin[K(x - x_r)], & x_r \leq x \leq x_r + nL_r \\ h_c, & x \geq x_r + nL_r \end{cases} \quad (23)$$

where A is the ripple amplitude, $L_r (=2\pi/K)$ is the ripple wavelength, n is the number of ripples, h_c is the water depth in the constant-depth region, K is the wave number of the ripple, and x_r is the x -coordinate of the starting point of the ripple patch. The grid size used is $\Delta x = L/30$ and $\Delta t = T/30$, where L is the wavelength and T is the period, respectively, of the surface waves. The computational domain for the numerical test is shown in Fig. 5, where L_{\max} is the wavelength on a flat bottom satisfying $2k/K = 0.5$.

Reflection coefficients calculated according to the proposed model and Kubo et al.'s model are given in Fig. 6 along with the experimental data. The result of the proposed model describes the resonant peak very well. However, the model of Kubo et al., while correctly positioning the resonant reflection, completely fails to predict its magnitude.

4.4. Transmission of random waves over a ripple patch

Linear dispersion properties of the type of the proposed model were verified by Kubo et al. (1992), who simulated the propagation of wave groups using their

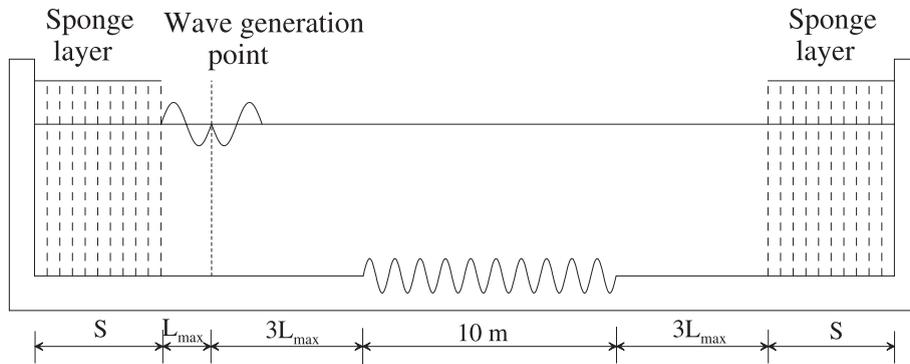


Fig. 5. Computational domain for numerical test of waves propagating over a ripple patch.

equation. Suh et al. (1997) made numerical tests for the transmission of unidirectional random waves over the ripple patch of Davies and Heathershaw (1984). In this study, we also conducted the same numerical tests.

The TMA shallow-water spectrum was used as a target spectrum (Bouws et al., 1985):

$$S(f) = \alpha g^2 (2\pi)^{-4} f^{-5} \exp[-1.25 (f/f_p)^{-4}] \times \gamma \exp[-(f/f_p - 1)^2 / 2\sigma^2] \phi_k(f, h) \quad (24)$$

where α is a spectral parameter, γ is the peak enhancement factor, σ is the spectral width parameter,

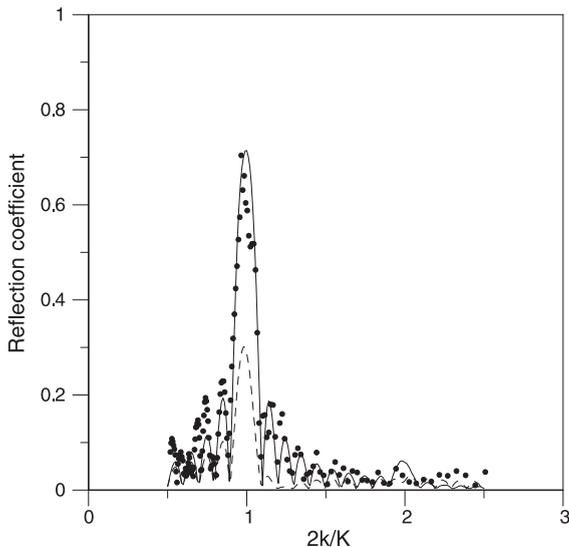


Fig. 6. Comparison of solutions of proposed model and Kubo et al.'s model, against experimental data of Davies and Heathershaw (1984); solid line = proposed model, dashed line = Kubo et al.'s model, filled circle = experimental data.

ter, and the Kitaigordskii shape function, $\phi_k(f, h)$ incorporates the effect of the finite water depth. Two cases of numerical tests were conducted: one with a narrow frequency spectrum ($\gamma=20$) and the other with a broad frequency spectrum ($\gamma=2$). $\alpha=7.57 \times 10^{-4}$, and the peak frequency of input wave, $f_p=0.767$ Hz were used in both cases. The computational domain was same as Fig. 5, where L_{max} was selected as the wavelength corresponding to the lowest frequency, and the width of sponge layer was $2.5L_{max}$. At the peak frequency of $f_p=0.767$ Hz, $2k_p/K$ is 1.0 and thus significant wave reflection from the ripple was expected near the peak frequency.

For the narrow spectrum case, the input spectrum was confined between 0.67 and 1.00 Hz, and for the broad spectrum case, the input spectrum was confined between 0.59 and 1.63 Hz. The carrier frequency \bar{f} was selected as 0.79 Hz for the narrow-banded spectrum and 0.98 Hz for the broad-banded spectrum by weight-averaging the input spectrum. In solving Eq. (7), the wave parameters were chosen to be the values corresponding to the carrier frequency. The water surface elevation was recorded at a distance $1.5L_{max}$ down-wave from the end of the ripple patch.

Fig. 7 shows a comparison of the transmitted wave spectra calculated by the proposed model and Kubo et al.'s model, against the finite element solution for the case of a narrow-banded frequency spectrum. As expected, the transmitted wave spectrum shows a significant reduction compared with the input spectrum near the Bragg resonant peak, whereas most of the wave energy is transmitted

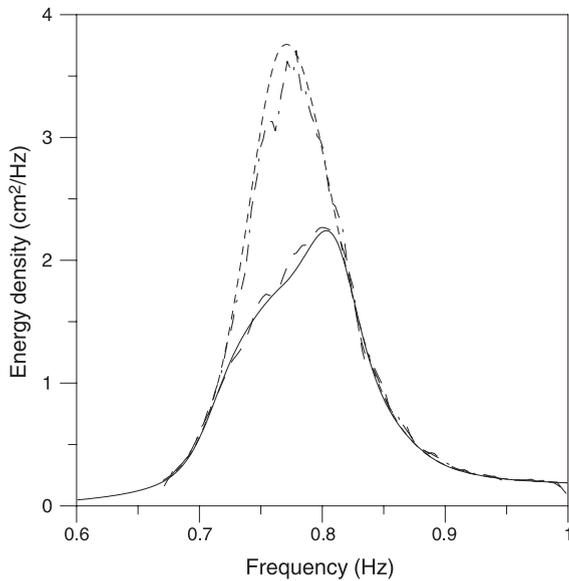


Fig. 7. Incident narrow-banded spectrum and transmitted spectra calculated by time-dependent equations with weight-averaged carrier frequency; short dashed line=incident wave, solid line=finite element model, long dashed line=proposed model, dash-dotted line=Kubo et al.'s model.

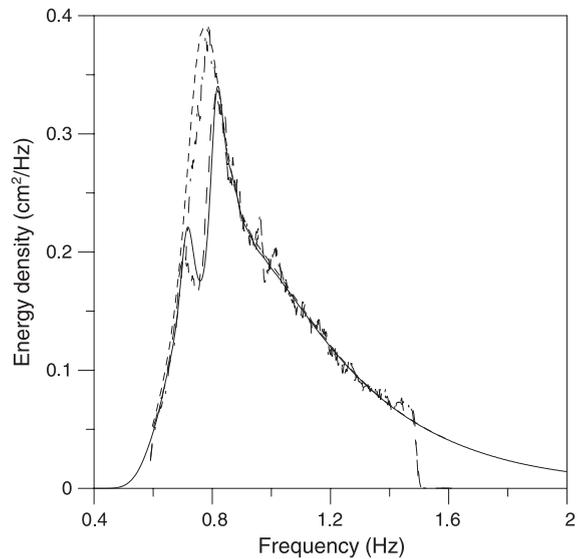


Fig. 8. Same as Fig. 7, but for broad-banded spectrum.

over the ripples for the wave components whose frequency is far from the Bragg resonant frequency. Notably, the solution by Kubo et al.'s model is far from the finite element solution, predicting a much larger wave transmission than the solution of the proposed model or the finite element solution. These results are nearly the same as those of Suh et al.'s model.

Fig. 8 shows a similar comparison for the case of a broad-banded frequency spectrum. As with the case of the narrow-banded spectrum, a reduction of the transmitted spectrum is observable near the Bragg resonant peak, and the transmitted spectrum is in good agreement with the finite element solution. The solution of the Kubo et al.'s model differs markedly from the finite element solution.

Suh et al. (1997) reported that accurate results could be obtained by dividing the frequency range into several bands and by modeling each of them with a representative carrier frequency. In particular, use of a single carrier frequency in the case of a broad-banded spectrum was not sufficient to obtain a reasonably accurate result. Fig. 9 shows a comparison of

the transmitted wave spectra calculated by the proposed model and the model of Suh et al. against the finite element solution for the case of a broad-banded

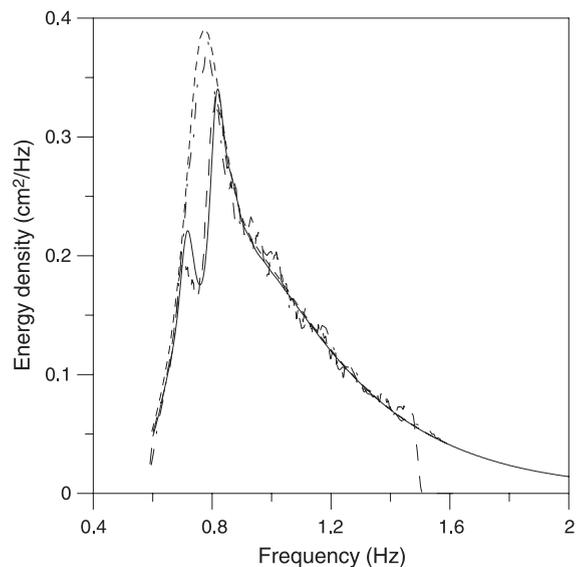


Fig. 9. Incident broad-banded spectrum and transmitted spectra calculated by time-dependent equations with weight-averaged carrier frequency; short dashed line=incident wave, solid line=finite element model, long dashed line=proposed model, dash-dotted line=Suh et al.'s model.

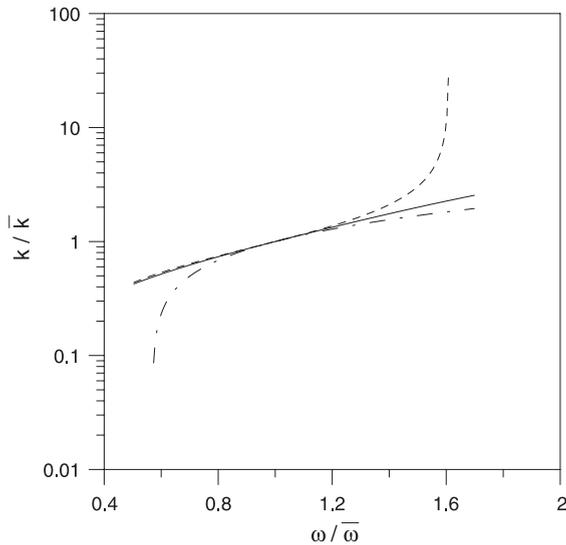


Fig. 10. Dispersion relation for $\bar{k}h=0.438\pi$; solid line=exact, dashed line=proposed model, dash-dotted line=Suh et al.'s model.

frequency spectrum when a single band spectrum is used. In contrast to the proposed model, the model of Suh et al. was not able to predict the reduction of the transmitted spectrum.

Fig. 10 shows the dispersion relations of Suh et al.'s model and the proposed model against the linear wave in the case of the broad-banded spectrum ($\bar{k}h=0.438\pi$). In the lower frequency range, the wave number of the proposed model is close to the exact one, whereas the wave number of Suh et al.'s model is smaller than the exact one and even diverges to negative infinity. On the other hand, in the higher frequency range, the wave number of Suh et al.'s model is more accurate than the proposed model and the wave number of the proposed model diverges to positive infinity. Because the dispersion relation of Suh et al.'s model does not agree with the exact one in lower frequency range, the Bragg reflection in lower frequency range cannot be predicted accurately whereas the proposed model does predict this reflection accurately.

5. Conclusions

A time-dependent wave model has been developed for waves propagating over rapidly varying topogra-

phy based on the extended mild-slope equation of Chamberlain and Porter (1995). Without the higher-order bottom effect terms, the resulting equation reduces to the time-dependent mild-slope equation developed by Kubo et al. (1992).

The proposed equation and Suh et al.'s (1997) equation were compared analytically in terms of linear dispersion. Suh et al.'s model is more accurate than the proposed model at a relative water depth shallower than 0.247π . When the relative water depth is deeper than 0.247π , the proposed model is more accurate in the lower frequency range, and Suh et al.'s model is more accurate in the higher frequency range.

Tests were made for the monochromatic wave reflections from a planar slope and periodic ripples. The results of the proposed model and Kubo et al.'s model were compared with those from the finite element method or experimental data. Of all the cases tested, the proposed model gave accurate results, whereas Kubo et al.'s model failed to predict the major characteristics of the phenomenon.

Tests were also made for the transmission of unidirectional random waves normally incident on a finite ripple patch. For both narrow and broad spectra, the solutions of the proposed model and Kubo et al.'s model were compared against the finite element solution. The comparisons demonstrated an improved accuracy of the proposed model than Kubo et al.'s model. For a broad spectrum, comparisons were made between the solutions of the proposed model and Suh et al.'s model. Using a single weight-averaged frequency as a carrier frequency, the proposed model gave more accurate solutions.

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